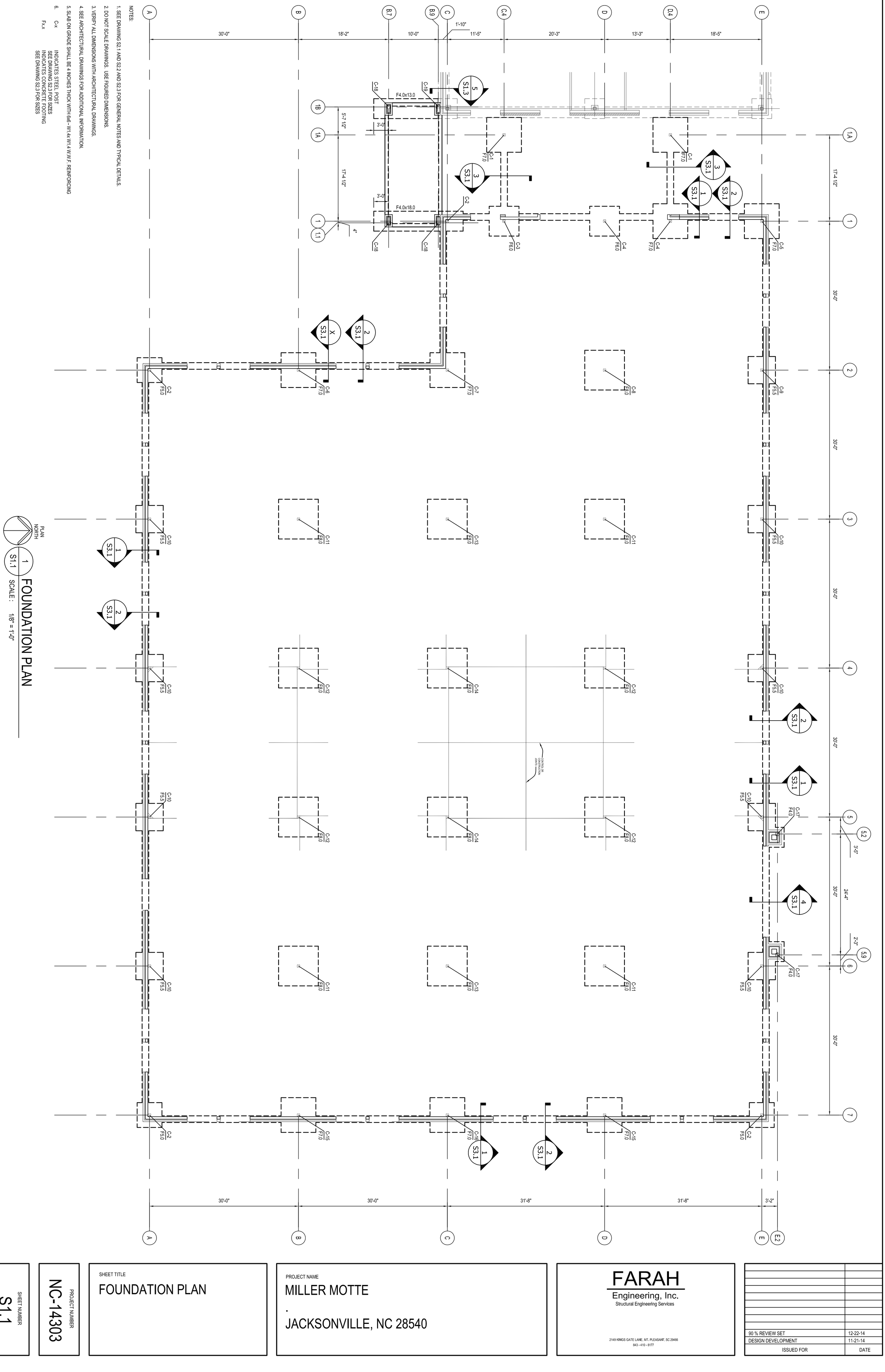
NOTES:

1. SEE DRAWING S2.1 AND S2.2 AND S2.3 FOR GENERAL NOTES AND TYPICAL DETAILS.
2. DO NOT SCALE DRAWINGS. USE FIGURED DIMENSIONS.
3. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
4. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
5. SLAB ON GRADE SHALL BE 4 INCHES THICK WITH 6x6 - W1.4xW1.4 W.W.F. REINFORCING
6. C-x INDICATES STEEL POST
   SEE DRAWING S2.3 FOR SIZES
   Fx.x INDICATES CONCRETE FOOTING
   SEE DRAWING S2.3 FOR SIZES



PLAN NORTH

1 S1.1

FOUNDATION PLAN

SCALE : 1/8" = 1'-0"

SHEET NUMBER S1.1

PROJECT NUMBER NC-14303

SHEET TITLE
FOUNDATION PLAN

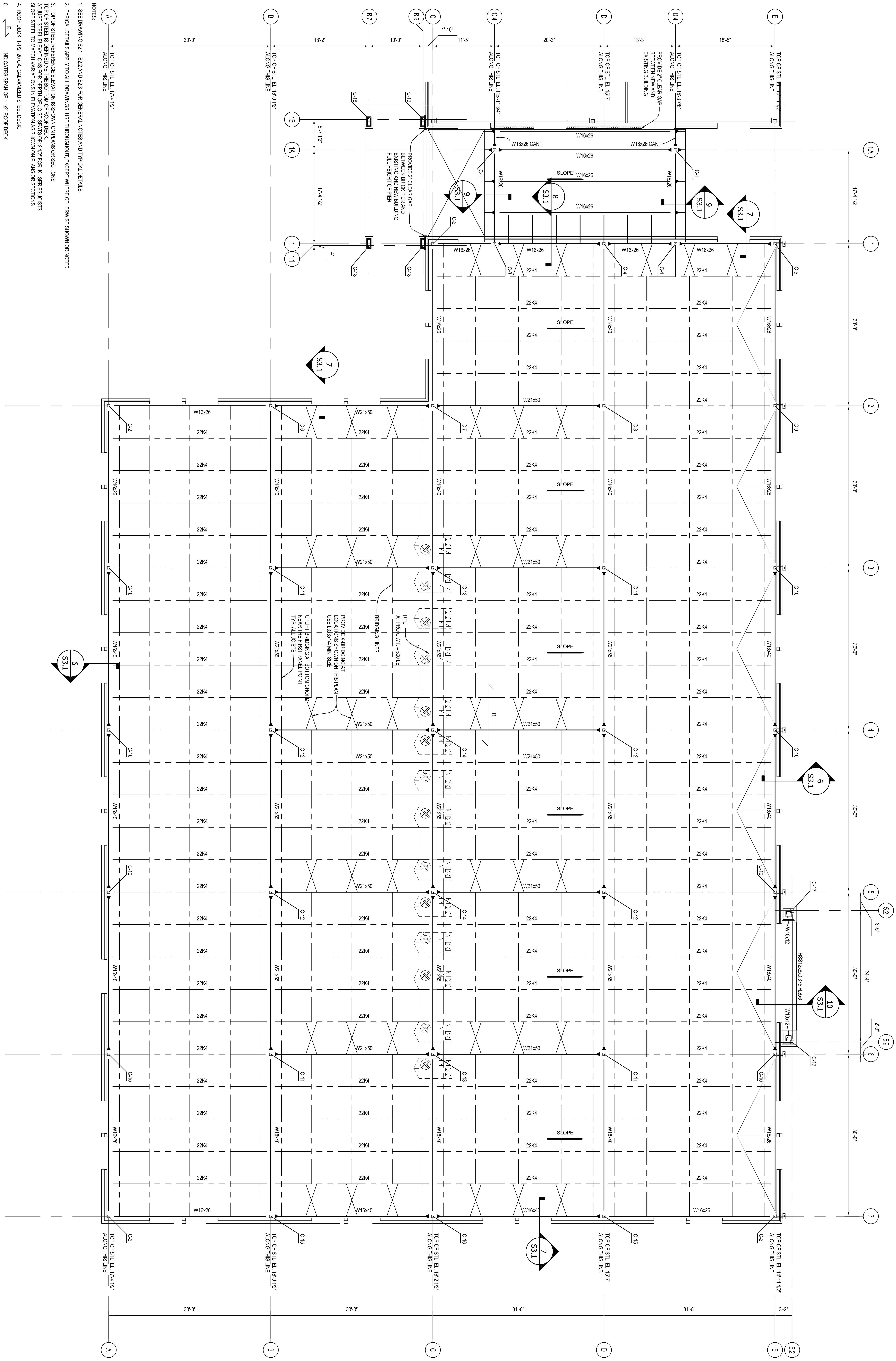
PROJECT NAME
MILLER MOTTE
.
JACKSONVILLE, NC 28540

FARAH
Engineering, Inc.
Structural Engineering Services
2149 KINGS GATE LANE, MT. PLEASANT, SC 29466
843 - 410 - 8177

ISSUED FOR	DATE
90 % REVIEW SET	12-22-14
DESIGN DEVELOPMENT	11-21-14

NOTES:

1. SEE DRAWING S2.1 - S2.2 AND S2.3 FOR GENERAL NOTES AND TYPICAL DETAILS.
2. TYPICAL DETAILS APPLY TO ALL DRAWINGS. USE THROUGHOUT, EXCEPT WHERE OTHERWISE SHOWN OR NOTED.
3. TOP OF STEEL REFERENCE ELEVATION IS SHOWN ON PLANS OR SECTIONS. TOP OF STEEL IS DEFINED AS THE BOTTOM OF ROOF DECK. ADJUST STEEL ELEVATIONS FOR DEPTH OF JOIST SEATS OF 2 1/2" FOR K- SERIES JOISTS. SLOPE STEEL TO MATCH VARIATIONS IN ELEVATION AS SHOWN ON PLANS OR SECTIONS.
4. ROOF DECK 1-1/2",20 GA. GALVANIZED STEEL DECK.
5. R INDICATES SPAN OF 1-1/2" ROOF DECK.
6. Cx INDICATES STEEL COLUMN SEE DRAWING S2.3 FOR SIZES
   ▼ INDICATES FULL MOMENT CONNECTION
6. MEMBERS ARE EQUALLY SPACED U.O.N.



1 S1.2 ROOF FRAMING PLAN
SCALE : 1/8" = 1'-0"

SHEET NUMBER
S1.2

PROJECT NUMBER
NC-14303

SHEET TITLE
ROOF FRAMING PLAN

PROJECT NAME
MILLER MOTTE
.
JACKSONVILLE, NC 28540

FARAH
Engineering, Inc.
Structural Engineering Services
2149 KINGS GATE LANE, MT. PLEASANT, SC 29466
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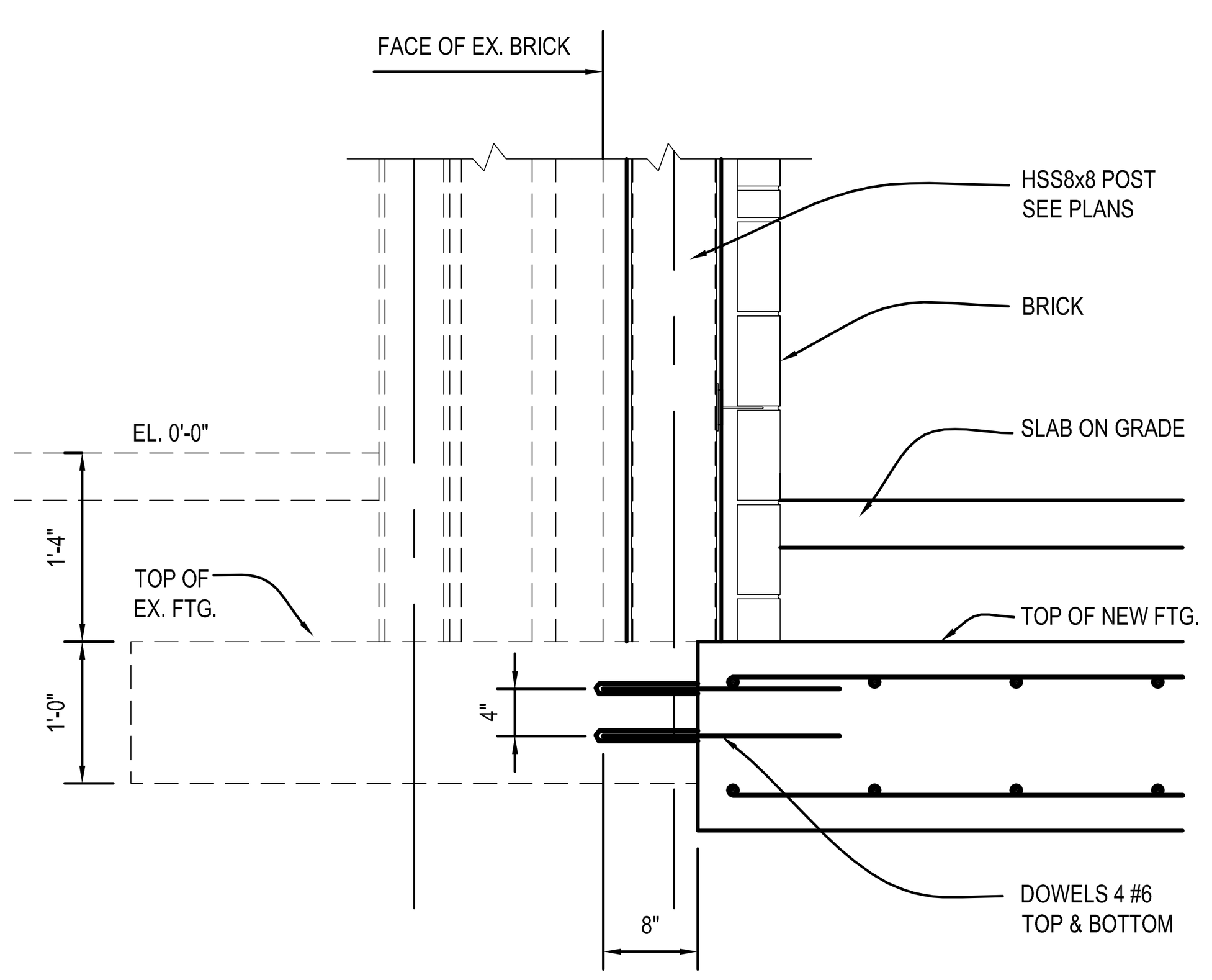
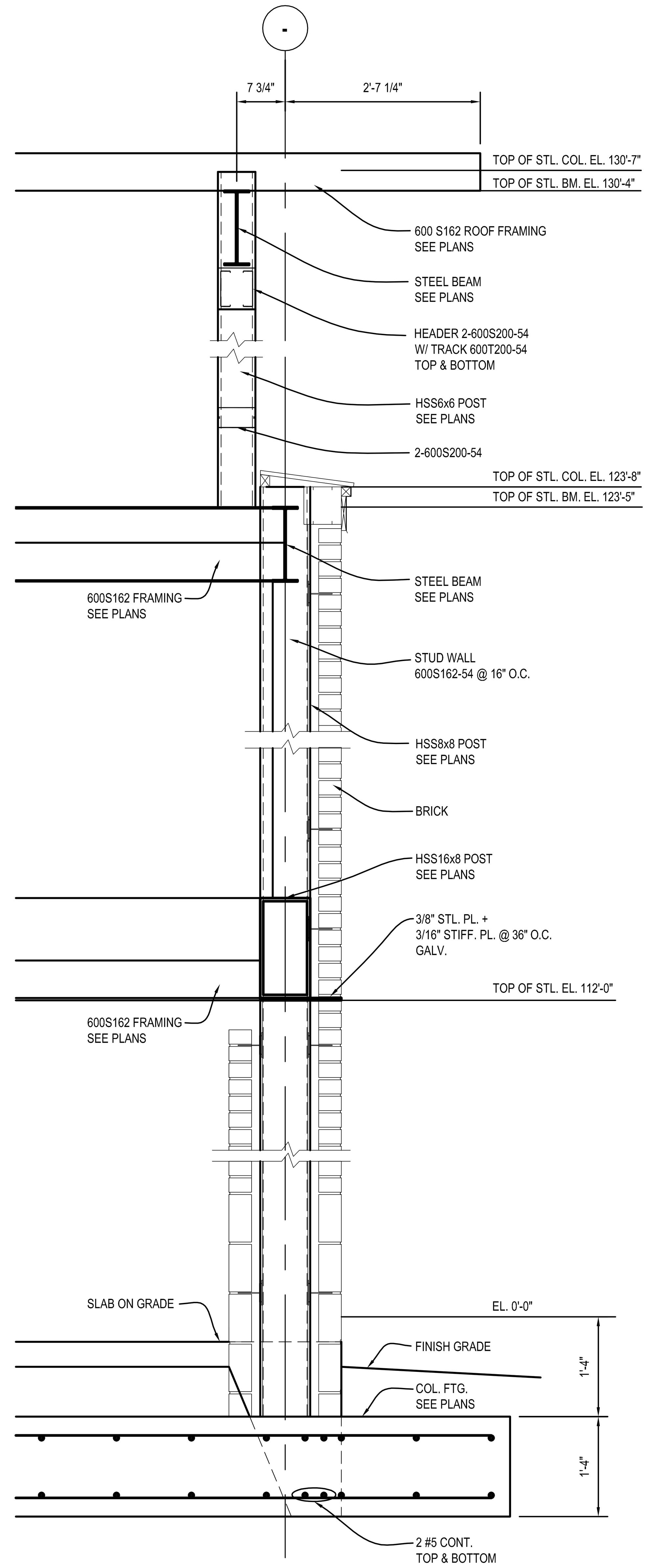
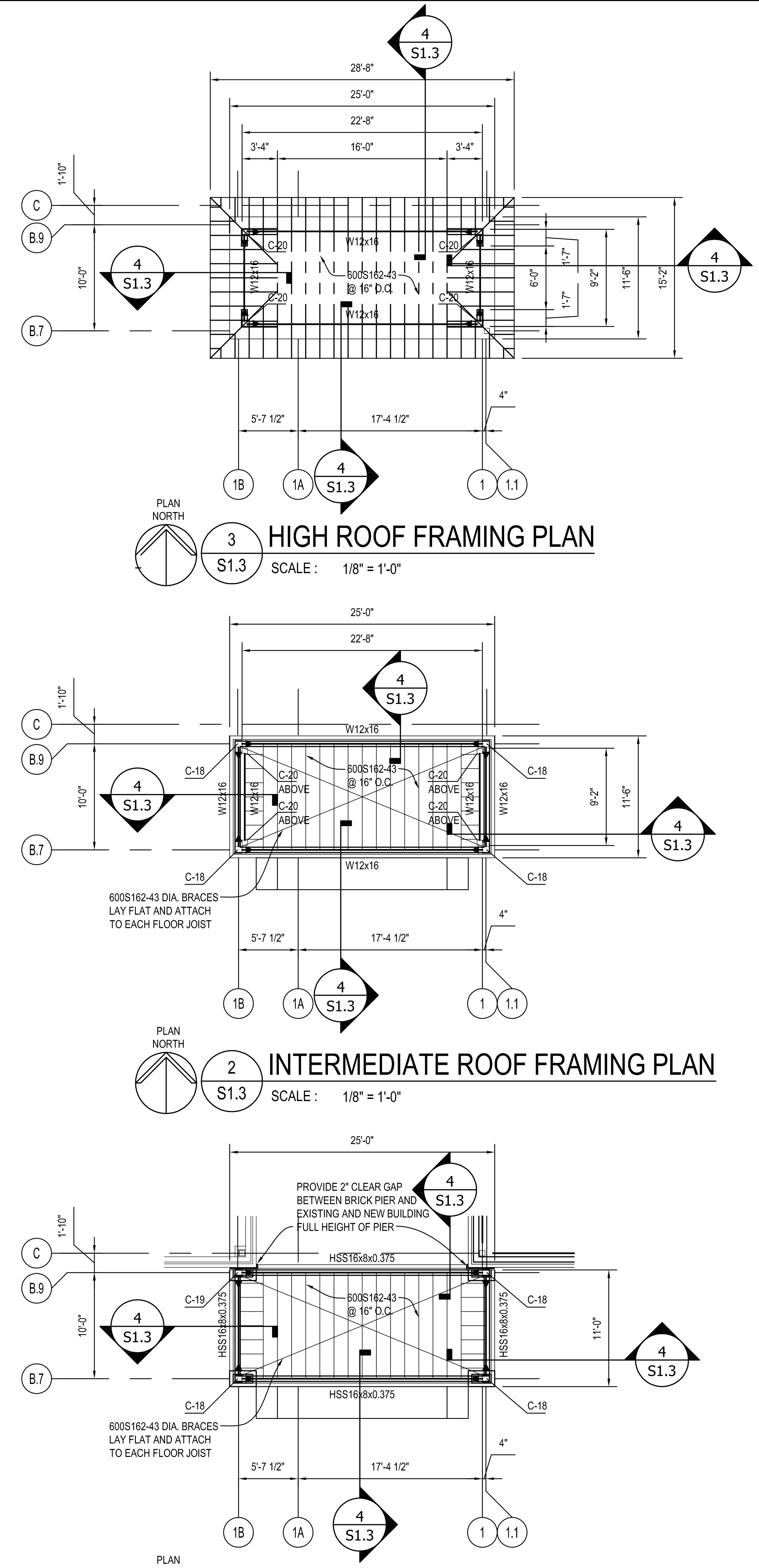


5
S1.3
SECTION
SCALE: 3/4"=1'-0"



4
S1.3
SECTION
SCALE: 3/4"=1'-0"



3
S1.3
HIGH ROOF FRAMING PLAN
SCALE : 1/8" = 1'-0"

2
S1.3
INTERMEDIATE ROOF FRAMING PLAN
SCALE : 1/8" = 1'-0"

1
S1.3
LOW ROOF FRAMING PLAN
SCALE : 1/8" = 1'-0"

NOTES:

1. SEE DRAWING S2.1 AND S2.2 AND S2.3 FOR GENERAL NOTES AND TYPICAL DETAILS.
2. DO NOT SCALE DRAWINGS. USE FIGURED DIMENSIONS.
3. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
4. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
5. C-x INDICATES STEEL POST SEE DRAWING S2.3 FOR SIZES

► INDICATES FULL MOMENT CONNECTION

90 % REVIEW SET	12-22-14
DESIGN DEVELOPMENT	11-21-14
ISSUED FOR	DATE

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PROJECT NAME
MILLER MOTTE
.
JACKSONVILLE, NC 28540

SHEET TITLE
ROOF FRAMING PLAN

PROJECT NUMBER
NC-14303

SHEET NUMBER
S1.3

GENERAL

1. ALL WORK SHALL BE PERFORMED IN STRICT ACCORDANCE WITH THE REQUIREMENTS OF THE GOVERNING MUNICIPAL CODES AND SPECIFICATIONS FOR THIS CONSTRUCTION PROJECT.

2. THE CONTRACTOR SHALL MAKE NO DEVIATION FROM DESIGN DRAWINGS WITHOUT WRITTEN APPROVAL OF THE ARCHITECT.

3. WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES, SHALL BE REPEATED.

4. IN ANY CASE OF CONFLICT BETWEEN NOTES, DETAILS, AND SPECIFICATIONS, THE MOST DEMANDING REQUIREMENTS SHALL GOVERN.

5. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND COORDINATION INVOLVED TO PROVIDE OPENINGS SHOWN ON THE ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.

6. CONTRACTOR SHALL VERIFY AND/OR ESTABLISH ALL EXISTING CONDITIONS AND DIMENSIONS AT THE SITE BEFORE ORDERING ANY MATERIAL AND COMMENCEMENT OF ANY WORK.

7. IF THE EXISTING FIELD CONDITIONS DO NOT PERMIT THE INSTALLATION OF THE WORK IN ACCORDANCE WITH THE DETAILS SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY AND PROVIDE A SKETCH OF THE CONDITION WITH HIS PROPOSED MODIFICATION OF THE DETAILS GIVEN ON THE CONTRACT DOCUMENTS. THE FINAL INSTALLATION SHALL BE DONE AS REQUIRED BY THE ARCHITECT AT NO ADDITIONAL COST TO THE OWNER.

8. CONTRACTOR SHALL PROVIDE FOR ANY DEWATERING AS REQUIRED DURING EXCAVATION AND CONSTRUCTION.

9. WHERE ALTERATIONS INVOLVE THE EXISTING SUPPORTING STRUCTURE, THE CONTRACTOR SHALL PROVIDE ALL SHORING AND PROTECTION REQUIRED TO INSURE THE STRUCTURAL INTEGRITY OF THE EXISTING STRUCTURE.

10. CONTRACTOR TO PROVIDE SHEETING, BRACING AND UNDERPINNING AS NECESSARY TO PREVENT ANY LATERAL OR VERTICAL MOVEMENTS OF EXISTING BUILDINGS, STREETS AND ANY EXISTING UTILITY LINES.

11. BRACING, SHEETING, SHORING, ETC., REQUIRED TO SUPPORT EXISTING BUILDINGS, SIDEWALKS, UTILITIES, ETC., SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER ENGAGED BY THE CONTRACTOR AND DEVELOP DETAILED DRAWINGS.

12. SHOP DRAWINGS FOR ALL MATERIALS TO BE SUBMITTED AND REVIEWED PRIOR TO START OF FABRICATION.

EARTHWORK

1. ALL EARTHWORK, INCLUDING EXCAVATIONS, FILLS, PROOFROLLING, TESTING, COMPACTION, ETC. TO BE PERFORMED IN ACCORDANCE WITH RECOMMENDATIONS CONTAINED IN SOILS REPORT AND THE SPECIFICATIONS.

2. BOTTOM OF FOOTING ELEVATIONS ARE APPROXIMATE AND ARE TO BE VERIFIED IN FIELD. IF FOUNDATION CONDITIONS PROVE TO BE UNACCEPTABLE AT ELEVATIONS SHOWN, FOOTINGS SHALL BE CARRIED DEEPER OR OTHER REDESIGN OF FOUNDATIONS WILL BE REQUIRED.

3. ALL ENGINEERED (CONTROLLED COMPACTED) FILL WITHIN THE BUILDING AREA SHALL BE CONSTRUCTED PRIOR TO ANY FOOTING EXCAVATION. SEE SPECIFICATIONS AND SOIL REPORT FOR REQUIREMENTS FOR CONSTRUCTION OF CONTROLLED COMPACTED FILL.

4. SEE SPECIFICATIONS AND SOIL REPORT FOR REQUIREMENTS FOR CONSTRUCTION OF CONTROLLED COMPACTED FILL. ALL FILL AND BACKFILL WITHIN THE BUILDING AREA, INCLUDING THAT FOR PLUMBING AND OTHER MECHANICAL WORK, SHALL BE COMPACTED IN LAYERS NOT EXCEEDING 6" LOOSE THICKNESS, WITH APPROVED MECHANICAL EQUIPMENT, AND IN ACCORDANCE WITH SPECIFICATIONS FOR COMPACTED FILL.

5. EXCAVATION AND CONSTRUCTION OF NEW FOOTINGS SHALL BE CONSTRUCTED SO AS NOT TO DISTURB EXISTING BUILDINGS, STREETS AND UTILITY LINES.

6. THE CONTRACTOR SHALL NOTIFY THE INSPECTING ENGINEER HIRED BY THE OWNER WHEN EXCAVATION OF THE BOTTOM OF THE FOOTINGS HAS BEEN COMPLETED. FOOTINGS SHALL NOT BE POURED UNTIL THE INSPECTING ENGINEER HAS VERIFIED THE BEARING VALUE.

7. ALL BACKFILL SHALL BE MECHANICALLY TAMPED TO 95% COMPACTION.

8. BACKFILL SHALL BE BROUGHT UP EQUALLY ON EACH SIDE OF WALLS AND GRADE BEAMS.

FOUNDATIONS

1. FOUNDATIONS ARE DESIGNED TO BEAR ON SUITABLE NATIVE NON-ORGANIC SOIL OR ON CONTROLLED COMPACTED ENGINEERED FILL HAVING A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 2000 POUNDS PER SQUARE FOOT.
THIS VALUE IS ASSUMED AND MUST BE VERIFIED IN FIELD.

2. ALL SLABS ON GROUND SHALL BEAR ON MECHANICALLY COMPACTED SOIL CAPABLE OF SUPPORTING 1000 P.S.F. DRAINAGE FILL UNDER SLABS SHALL BE COMPACTED SAND AND GRAVEL OR CRUSHED STONE AS REQUIRED BY THE EXCAVATION. ALL UN-ENGINEERED FILL, TOP SOIL. ORGANIC AND VEGETATIVE MATERIAL SHOULD BE REMOVED PRIOR TO THE PLACEMENT OF ENGINEERED FILL.

3. NO FOOTINGS SHALL BE PLACED IN WATER OR ON FROZEN GROUND.

4. PROVIDE FROST PROTECTION FOR FOOTING AND AREA WITHIN 3 FEET OF THE FOOTING PERIMETER. PROTECT FOOTINGS IN ORDER TO PREVENT FREEZING AND HEAVING OF THE BEARING STRATUM.

5. FINISHED EXCAVATIONS AND BEARING GRADES SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL INSPECTION AGENCY BEFORE ANY CONCRETE IS PLACED.

6. NO BACKFILLING AGAINST FOUNDATION WALLS AND GRADE BEAMS SHALL BE DONE UNTIL CONCRETE HAS ATTAINED 75% OF ITS 28 DAY STRENGTH. BEFORE BACKFILLING, PROVIDE BRACING FOR WALLS OR WALL BUTTRESSES AND GRADE BEAMS SUSTAINING MORE THAN 2'-0" OF UNBALANCED EARTH PRESSURE. THIS BRACING IS TO REMAIN UNTIL THE PERMANENT RESTRAINTS BECOME EFFECTIVE.

7. UNDERGROUND UTILITIES OR PIPES SHALL NOT BE PLACED BELOW FOOTINGS. IF ANY SUCH CONDITION OCCURS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND DROP THE BOTTOM OF FOOTING IN ORDER TO CLEAR PIPE.

8. WHERE NEW FOOTINGS ABUTS EXISTING FOUNDATIONS, CAREFULLY HAND EXCAVATE AND DETERMINE BOTTOM OF EXISTING FOOTING. IF DIFFERENT THAN ANTICIPATED, ADJUST NEW FOUNDATIONS TO MATCH EXISTING. IN NO CASE SHALL THE NEW FOOTING BE LOWER THAN THE EXISTING WITHOUT WRITTEN APPROVAL FROM ARCHITECT / ENGINEER.

CONCRETE

1. ALL CONCRETE SHALL BE REINFORCED, DETAILED, AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI-318).

2. ALL INTERIOR AND FOUNDATION CONCRETE (NORMAL CONCRETE) SHALL HAVE A MINIMUM COMPRESSIVE 28 DAY STRENGTH OF 3,000 PSI WITH 3 INCHES MAXIMUM SLUMP. EXTERIOR CONCRETE SHALL BE MINIMUM 4000 PSI WITH AIR ENTRAINMENT 4% TO 6%.

3. REINFORCING STEEL SHALL BE A-615 GRADE 60 (60 KSI) DEFORMED HI-BOND AND CONFORM TO THE LATEST ASTM SPECIFICATIONS.

4. WELDED WIRE FABRIC: (W.W.F.) ASTM A-185. WELDED WIRE FABRIC SHALL BE FURNISHED AND PLACED IN FLAT SHEETS. INSTALL IN AS LONG LENGTHS AS PRACTICABLE. LAP ADJOINING PIECES AT LEAST ONE FULL MESH AND LACE SPLICES WITH WIRE. OFFSET END LAPS IN ADJACENT WIDTHS TO PREVENT CONTINUOUS LAPS IN EITHER DIRECTION.

5. RETAIN A TESTING AGENCY, ACCEPTABLE TO THE OWNER, WHO WILL PERFORM INSPECTION AND TESTING OF CONCRETE. THE TESTING AGENCY SHALL SUBMIT, IN A TIMELY MANNER, THE RESULTS OF THEIR INSPECTION AND TEST TO THE ARCHITECT FOR REVIEW.

6. BEFORE PLACING ANY CONCRETE, SUBMIT MIX DESIGNS TO ARCHITECT FOR APPROVAL.

7. SHOP DRAWINGS SHOWING REINFORCING DETAILS INCLUDING BAR SIZES, SPACING AND PLACEMENT SHALL BE SUBMITTED AND REVIEWED PRIOR TO FABRICATION.

8. CONTRACTOR TO RECOGNIZE THE POTENTIAL PROBLEM OF CURLING AND SHRINKAGE CRACKING WHEN CASTING SLABS ON VAPOR BARRIER AND/OR WET SUBGRADE. EVERY EFFORT MUST BE MADE TO USE PROPER MIX, ADMIXTURES WITH LOW SHRINKAGE POTENTIAL AND SLUMP AS SPECIFIED. AFTER CASTING, MAINTAIN THE MOISTURE CONTENT OF THE SLAB AS UNIFORM AS POSSIBLE, AND DO NOT ALLOW THE TOP OF THE SLAB TO BECOME DRIER THAN THE BOTTOM. CURE SLABS AS SPECIFIED USING A HIGH QUALITY CURING COMPOUND OR BY MOIST CURING AS APPLICABLE.

9. MINIMUM CONCRETE COVER SHALL BE (UNLESS OTHERWISE NOTED):

A. UNFORMED SURFACES IN CONTACT WITH GROUND (FOOTING BOTTOMS). — 3 INCHES

B. SLABS ON GRADE — 1" TOP COVER

C. FORMED SURFACES IN CONTACT WITH GROUND OR EXPOSED TO THE WEATHER (GRADE BEAMS, WALLS, ETC.) — 2 INCHES

D. IN ALL CASES, CLEARANCE NOT LESS THAN THE DIAMETER OF THE BARS.

10. WHERE CONTINUOUS BARS ARE CALLED FOR, THEY SHALL BE RUN CONTINUOUSLY AROUND CORNERS AND LAPPED AT NECESSARY SPLICES, AND HOOKED AT DISCONTINUOUS ENDS.

11. ALL FOOTINGS SHALL HAVE DOWELS THE SAME SIZE AND NUMBER AS VERTICAL REINFORCING IN WALLS, PIERS OR COLUMNS ABOVE, AND SHALL EXTEND A MINIMUM OF 32 BAR DIAMETERS ABOVE THE TOP OF FOOTING.

12. PROVIDE CORNER BARS IN ALL EXTERIOR CORNERS OF GRADE BEAMS, AND WALLS, TO MATCH HORIZONTAL REINFORCING.

13. LAP SPLICES, DEVELOPMENT LENGTHS AND DETAILS FOR REINFORCING BARS SHALL CONFORM TO THOSE LISTED IN THE AMERICAN CONCRETE INSTITUTE SPECIFICATIONS (LATEST EDITION).

14. COLUMN FOOTINGS SHALL BE CENTERED ON COLUMN CENTERLINES OR COLUMN BASE PLATE CENTERLINES UNLESS NOTED OTHERWISE.

15. CLEAN LOOSE RUST AND MILL SCALE, EARTH, ICE AND OTHER MATERIAL THAT MAY REDUCE THE CONCRETE BOND TO REINFORCEMENT.

16. DO NOT USE SUPPORTS OF REINFORCING AS BASIS FOR RUNWAYS FOR CONCRETE CONVEYING EQUIPMENT AND SIMILAR CONSTRUCTION LOADS.

17. COORDINATE THE INSTALLATION OF JOINT MATERIAL AND MOISTURE BARRIERS WITH THE PLACEMENT OF FORMS AND REINFORCING STEEL.

18. BEFORE CONCRETE IS POURED, MECHANICAL AND ELECTRICAL CONTRACTORS SHALL VERIFY LOCATIONS AND SIZES OF ALL OPENINGS, PADS, AND SLEEVES REQUIRED BY THEIR WORK.

19. POURING OF CONCRETE SHALL NOT START UNTIL THE REINFORCING HAS BEEN PLACED AND APPROVED BY THE INSPECTING ENGINEER.

20. FOR COLD AND HOT WEATHER CONSTRUCTION, FOLLOW THE PROCEDURES INDICATED IN 2009 IBC CODE (SECTION 1905.12 AND 1905.13).

SLAB ON GROUND

1. PROVIDE CONSTRUCTION, CONTROL AND ISOLATION JOINTS IN SLABS ON GRADE INCLUDING ISOLATION JOINTS AT COLUMNS, AS NOTED ON DRAWINGS.

2. MAXIMUM SPACING OF CONTROL JOINTS FOR SLAB ON GRADE TO BE 15 FEET.

3. SAWED CONTROL JOINTS MAY BE USED IN LIEU OF FORMED CONTROL JOINTS FOR SLAB ON GRADE. CUT JOINT AS SOON AS POSSIBLE AFTER FLOOR HAS BEEN PLACED.

CONCRETE MASONRY

1. MASONRY SHALL BE IN ACCORDANCE WITH THE LATEST:
-BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
(ACI 530/ASCE 5/ TMS 402)
-SPECIFICATIONS FOR MASONRY STRUCTURES
(ACI 530.1/ASCE 6/ TMS 602).

2. MORTAR SHALL BE PORTLAND CEMENT MORTAR IN ACCORDANCE WITH ASTM C 270, TYPE S.

3. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476. GROUT STRENGTH SHALL BE f'c = 2500 PSI MIN.

4. MINIMUM MASONRY STRENGTH SHALL BE f'm = 2000 PSI.

5. REINFORCEMENT: ASTM A 615 GRADE 60.

6. PROVIDE THE FOLLOWING LINTELS FOR EACH 4" WYTHE OF MASONRY UNLESS NOTED OTHERWISE:

L4x3 1/2x3/8 (LLV)
WITH 6" BEARING EACH END TYPICAL FOR OPENINGS UP TO 4'-4"

L5x3 1/2x3/8 (LLV)
WITH 6" BEARING EACH END TYPICAL FOR OPENINGS UP TO 6'-8"

L6x3 1/2x3/8 (LLV)
WITH 8" BEARING EACH END TYPICAL FOR OPENINGS UP TO 8'-0"

ALL BRICK LOOSE LINTELS MUST BE GALVANIZED

STRUCTURAL STEEL

1. SHOP DETAILS, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE LATEST AISC:
-SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS (AISC 360)
-SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS INCLUDING SUPPLEMENT NO 1 (AISC 341).

2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

A. W- SHAPE STEEL: ASTM A-992 (Fy = 50 KSI)
B. STEEL PIPE: ASTM A-53, GRADE B (Fy = 35 KSI)
C. TUBULAR SHAPES: ASTM A500, GRADE B (Fy = 46 KSI)
D. ALL OTHERS ASTM A36 (Fy = 36 KSI)

3. ALL BOLTED CONNECTIONS SHALL BE WITH ASTM A325 HIGH STRENGTH BOLTS.

4. ANCHOR BOLTS SHALL CONFORM TO ASTM A-307.

5. ALL WELDING SHALL BE DONE WITH APPROPRIATE E70 SERIES ELECTRODES COMPATIBLE WITH THE NEW AND EXISTING STEEL AND SHALL CONFORM TO THE REQUIREMENTS OF THE "CODE FOR WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY.

6. WELDING OF REINFORCING BARS TO OTHER BARS OR STRUCTURAL STEEL: E90XX ELECTRODE.

7. GAS CUTTING OF MAIN STRUCTURAL MEMBERS IN THE FIELD WILL NOT BE PERMITTED.

8. ALL STEEL SHALL BE FABRICATED AND ERECTED WITH NATURAL CAMBER UP UNLESS NOTED OTHERWISE.

9. ALL BEAMS SHALL BE EQUALLY SPACED UNLESS NOTED OTHERWISE.

10. SHOP AND ERECTION DRAWINGS MUST SHOW ALL SHOP AND FIELD WELDS.

11. MAIN SUPPORT MEMBERS FOR THE METAL DECK ARE SHOWN. DURING PREPARATION, SUBMISSION AND REVIEW OF SHOP DRAWINGS, ANY ADDITIONAL ANGELS OR MISCELLANEOUS ATTACHMENT DETAILS REQUIRED TO SUPPORT THE METAL DECK AT THE REQUIRED ELEVATION SHALL BE PROVIDED BY THE STRUCTURAL STEEL CONTRACTOR.

12. ALL STEEL ANGLES AND PLATES ALONG WITH BOLTS AND WASHERS, IN DIRECT CONTACT WITH EXTERIOR FINISH MASONRY AND EXPOSED STRUCTURAL STEEL, SHALL BE GALVANIZED.

13. ALL SPANDRELS ADJACENT TO MASONRY SHALL HAVE MASONRY TIES.

14. GUYS AND OTHER BRACING REQUIRED TO PROVIDE LATERAL STABILITY TO STEEL FRAME SHALL BE ADEQUATELY SIZED AND ANCHORED. THIS BRACING SHALL REMAIN UNTIL PERMANENT BRACING ELEMENTS ARE INSTALLED.

15. CONTRACTOR SHALL USE RIGID TEMPLATE TO INSTALL ANCHOR BOLTS.

16. ANCHOR BOLTS, BASE PLATES OR BEARING PLATES SHALL BE LOCATED AND BUILT INTO CONNECTING WORK, PRE-SET BY TEMPLATES OR SIMILAR METHOD. PLATES SHALL BE SET IN FULL BEDS OF NON-SHRINK MORTAR OR GROUT.

17. ALL EXPOSED WELDED SURFACES SHALL BE SMOOTH GROUND AND COATED WITH APPROPRIATE PRIMER/PAINTS AS SPECIFIED.

18. FRAMING CONNECTIONS: ALL PROVISIONS OF THE LATEST "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS", SHALL GOVERN. ALL CONNECTIONS SHALL BE "FRAMED BEAM CONNECTIONS" DESIGNED IN ACCORDANCE WITH PART 4 OF THE AISC MANUAL.

19. THE CONNECTIONS SHALL BE DESIGNED FOR END REACTIONS AS INDICATED ON UNIFORM LOAD CONSTANTS FOR BEAMS LATERALLY SUPPORTED TABLES IN PART 2 OF THE AISC MANUAL. ALL CONNECTION CAPACITIES TO BE GREATER THAN 6 KIPS.

20. REFER TO ARCHITECTURAL DRAWINGS FOR STEEL PLATES, ANGLES, ETC., ATTACHED TO BEAMS, FRAMES, ETC., FOR SUPPORT OF FASCIA AND OTHER CONSTRUCTION.

21. THE CONTRACTOR IS RESPONSIBLE FOR THE ERECTION SAFETY OF ALL STEEL CONNECTIONS, INCLUDING BUT NOT LIMITED TO: CONFIGURATION, SEQUENCE, USE OF: BLOCKING, EXTENDED CLIP ANGLES, CLAMPS, ETC.

22. SHOP DRAWING REVIEW IS ONLY TO VERIFY LOAD CARRYING CAPACITY.

23. STEEL AND CONNECTIONS ABOVE THE ROOF STRUCTURE (STEEL EXPOSED TO WEATHER) SHALL BE CLEANED PER SSPC-SP6 AND SHOP PRIME PAINTED WITH 2-3 MILS DRY FILM THICKNESS OF SERIES 65 EPOXI-PRIME BY TNEMEC. FIELD FINISH PAINT SHALL BE 2-3 MILS DRY FILM THICKNESS EACH OF TNEMEC SERIES 66 EPOXOLINE SECOND COAT AND TNEMEC SERIES 73 ENDURASHIELD FINISH COAT. FINISH COLOR SHALL BE SELECTED PER OWNER'S STANDARD. SECOND COAT SHALL BE TINTED TO MATCH FINISH COAT.

24. STEEL AND CONNECTIONS BELOW THE ROOF STRUCTURE (NOT EXPOSED TO WEATHER) SHALL BE CLEANED PER SSPC-SP3 AND SHOP PRIME PAINTED WITH TNEMEC #99 PRIMER. FIELD FINISH PAINT WITH TWO COATS OF TNEMIC SERIES 23 ENDURATONE SEMI-GLOSS, COLOR PER OWNER'S STANDARD.

METAL DECK

1. STEEL ROOF DECK SHALL BE WIDE RIB DECK COMPLYING WITH LATEST SPECIFICATIONS OF THE SDI. SHEETS TO BE AS LARGE AS POSSIBLE.

2. ALL STEEL ROOF DECK TO BE CONTINUOUS OVER 3 SPANS (MINIMUM).

STEEL JOIST AND JOIST GIRDERS

1. STEEL JOIST AND JOIST GIRDERS SHALL BE DESIGNED, DETAILED WITH BRACING, BRIDGING, ETC., FABRICATED AND ERECTED IN ACCORDANCE WITH STEEL JOIST INSTITUTE SPECIFICATIONS.

2. MANUFACTURERS SHALL BE MEMBERS OF THE STEEL JOIST INSTITUTE.

3. ALL JOISTS TO RECEIVE SHOP COAT OF FABRICATOR'S STANDARD NON-BITUMINOUS PRIMER.

4. PROVIDE JOIST BRIDGING IN ACCORDANCE WITH SJI REQUIREMENTS INCLUDING UPLIFT CONSIDERATIONS UNLESS MORE STRINGENT CONDITIONS ARE SHOWN

A. PROVIDE BRIDGING AT FIRST BOTTOM CHORD PANEL POINT FROM THE SUPPORT.

5. ATTACH JOISTS ON COLUMN LINES WITH ERECTION BOLTS AND WELD AFTER PLUMBING ALIGNING. ATTACH ALL OTHER JOISTS TO STEEL BY WELDING.

6. EXTEND JOIST BOTTOM CHORDS AT ALL LOCATIONS INDICATED ON PLANS. CONNECT BOTTOM CHORD EXTENSIONS AFTER SLABS OR ROOF BALLAST HAVE BEEN PLACED.

7. CONCENTRATED LOADS DUE TO HANGERS, HEADERS, MECHANICAL EQUIPMENT, ETC. SHALL BE SUPPORTED ONLY AT JOIST PANEL POINTS. WHERE SUPPORT POINTS DO NOT ALIGN WITH JOIST PANEL POINTS, PROVIDE SUPPLEMENTAL MEMBERS FROM SUPPORT POINTS TO CHORD MEMBER PANEL POINTS.

8. PROVIDE 13 PSF MINIMUM NET UPLIFT CAPABILITIES.

LIGHT GAGE METAL FRAMING

1. STANDARDS

A. WORK SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS:

1. AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS"

2. AWS D1.3 "STRUCTURAL WELDING CODE-SHEET STEEL"

3. ASTM C955 "STANDARD SPECIFICATION FOR LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS, RUNNERS (TRACKS), AND BRACING OR BRIDGING FOR SCREW APPLICATION OF GYPSUM BOARD AND METAL PLASTER BASES."

4. ASTM C1007 "STANDARD SPECIFICATION FOR INSTALLATION OF LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS AND RELATED ACCESSORIES.

5. ALL PERTINENT FEDERAL, STATE AND LOCAL CODES.

B. THE MOST STRINGENT REQUIREMENTS SHALL GOVERN IN CONFLICTS BETWEEN SPECIFIED CODES AND STANDARDS

2. STUD MANUFACTURER MUST BE MEMBERS IN GOOD STANDING OF THE STEEL STUD MANUFACTURERS ASSOCIATION. STUD SIZES SHALL BE AS SPECIFIED BY STEEL STUD MANUFACTURER'S ASSOCIATION (SSMA)

3. ALL FRAMING MEMBERS SHALL BE OF THE TYPE, SIZE AND GAGE AS SHOWN ON THE PLANS AND SECTIONS:

4. MATERIALS

A. GALVANIZED MATERIAL

1. ALL MATERIAL SHALL BE GALVANIZED. ALL FRAMING MATERIALS SHALL BE FORMED FROM STEEL MEETING THE MINIMUM REQUIREMENTS OF THE AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS"

2. ALL FRAMING MATERIALS SHALL HAVE A GALVANIZED COATING MEETING THE MINIMUM REQUIREMENTS OF ASTM C955.

B. PROPERTIES: THE PHYSICAL AND STRUCTURAL PROPERTIES LISTED BY STEEL STUD MANUFACTURER'S ASSOCIATION (SSMA) SHALL BE CONSIDERED THE MINIMUM PERMITTED FOR ALL FRAMING MEMBERS. SPECIFICALLY THE FOLLOWING MEMBER PROPERTIES, CALCULATED IN ACCORDANCE WITH THE AISI SPECIFICATION SHALL BE PROVIDED: Ix (MOMENT OF INERTIA), Sx(SECTION MODULUS), AREA, Rx(RADIUS OF GYRATION), Fy(YIELD STRENGTH), Mr(RESISTING MOMENT).

5. YIELD STRENGTH:

A. ALL CR RUNNERS AND 20-GA AND 18-GA STUDS OR JOIST MEMBERS: 33,000 PSI

B. ALL 16-GA, 14-GA AND 12-GA STUD OR JOIST MEMBERS: 50,000 PSI

6. PRIOR TO PREFABRICATION OF FRAMING, THE CONTRACTOR SHALL SUBMIT FABRICATION AND ERECTION DRAWINGS TO THE ARCHITECT OR ENGINEER FOR REVIEW.

7. ENGAGE A FABRICATOR OR MANUFACTURER WHO EMPLOYS A QUALIFIED PROFESSIONAL ENGINEER, LICENSED IN THE STATE OF THE MICHIGAN, TO PREPARE DESIGN CALCULATIONS, SHOP DRAWINGS, AND OTHER STRUCTURAL DATA.

8. AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER, WHICH WILL ASSURE THAT ENDS OF THE STUDS ARE POSITIONED AGAINST THE INSIDE TRACK WEB, PRIOR TO STUD AND TRACK ATTACHMENT.

9. STUDS SHALL BE PLUMBED, ALIGNED AND SECURELY ATTACHED TO THE FLANGES OF BOTH UPPER AND LOWER TRACKS.

10. BOTH STUD FLANGES MUST BE ATTACHED TO TOP AND BOTTOM RUNNER FLANGES TO PROVIDE PROPER END SUPPORT.

11. WALL STUD BRIDGING SHALL BE ATTACHED IN A MANNER TO PREVENT STUD ROTATION.

12. STUDS MUST BE LATERALLY BRACED WITH MECHANICAL BRACING INSTALLED AT A MAXIMUM OF 48" O.C. FOR BRACING DETAILS REFER TO STEEL STUD MANUFACTURER'S STANDARD DETAILS. ASSOCIATION (SSMA)

13. JOISTS SHALL BE LOCATED DIRECTLY ON BEARING STUDS OR A LOAD DISTRIBUTION MEMBER TO BE PROVIDED AT THE TOP TRACK. ALL STUDS MUST HAVE FULL BEARING SURFACE SQUARELY ON BOTTOM TRACK.

14. INSTALL HEADERS OVER WALL OPENINGS WIDER THAN THE STUD SPACING. LOCATE HEADERS ABOVE OPENINGS AS INDICATED ON THE PLANS. FABRICATE HEADERS IN COMPLIANCE WITH THE APPROVED SHOP DRAWINGS.

15. FRAME WALL OPENINGS WITH NOT LESS THAN A DOUBLE STUD AT EACH JAMB OR FRAME AS INDICATED ON THE APPROVED SHOP DRAWINGS.

16. TEMPORARY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED.

17. SPLICES IN AXIALLY LOADED STUDS SHALL NOT BE PERMITTED.

DESIGN LOADS

1. DESIGN CODE — NC BC 2012

2. ROOF TRUSSES DESIGN LOADS

	DEAD LOAD	LIVE LOAD
TOP CHORD	15 PSF	20 PSF
BOTTOM CHORD	10 PSF	-- PSF

3. DESIGN WIND LOAD FOR STRUCTURAL FRAME:

A. WIND IMPORTANCE FACTOR	Iw = 1.15
B. EXPOSURE	B
C. BASIC WIND SPEED	VfS = 125 MPH
D. WIND DIRECTIONAL FACTOR "Kd"	0.85
E. TOPOGRAPHIC FACTOR "Kzt"	1.00
F. GUST EFFECT FACTOR "G"	0.85
G. INTERNAL PRESSURE COEFFICIENT "GCpi"	0.18 +/-

H. CALCULATED BASE SHEAR:
Vx = 57 KIPS
Vy = 94 KIPS

4. SNOW LOADS (ASCE 7):

A. GROUND SNOW LOAD, "Pg"	10 PSF
B. SNOW EXPOSURE FACTOR, "Ce"	1.0
C. SNOW LOAD IMPORTANCE FACTOR, "I"	1.1
D. THERMAL FACTOR, "Ct"	1.0

5. SEISMIC LOADS (ASCE 7-05)

A. MAPPED SPECTRAL ACCELERATIONS FOR SHORT PERIODS:
Ss = 0.204 g
Fa = 1.6
Sds = 0.217

B. MAPPED SPECTRAL ACCELERATIONS FOR 1-SECOND PERIODS:
S1 = 0.076 g
Fv = 2.4
Sd1 = 0.122

C. SITE CLASS — D
E. SEISMIC DESIGN CATEGORY "SDC" — B
F. BASIC STRUCTURAL SYSTEM - STEEL MOMENT FRAMES
-SEISMIC OCCUPANCY IMPORTANCE FACTOR: Ie = 1.25
-RESPONSE MODIFICATION FACTOR: R = 3.0
-OVERSTRENGTH FACTOR Ωo = 3.0
-REDUNDANCY FACTOR ρ = 1.0
-DEFLECTION AMPLIFICATION FACTOR Cd = 3.0
-ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE PROCEDURE
-CALCULATED BASE SHEAR:
Vx = 54 KIPS
Vy = 54 KIPS

6. BUILDING IS NOT DESIGNED FOR FUTURE VERTICAL OR HORIZONTAL EXPANSION.

ISSUED FOR	DATE
90 % REVIEW SET	12-22-14
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PROJECT NAME
MILLER MOTTE
.
JACKSONVILLE, NC 28540

SHEET TITLE
GENERAL NOTES

PROJECT NUMBER
NC-14303

SHEET NUMBER
S2.1

SPECIAL INSPECTION NOTES:

1. SPECIAL INSPECTION SHALL BE PERFORMED IN ACCORDANCE WITH 2012 NORTH CAROLINA (INTERNATIONAL) BUILDING CODE.

2. CONTRACTOR MUST COORDINATE AND SCHEDULE ALL INSPECTION AND TESTING REQUIRED IN EACH SECTION OF THE SPECIFICATION AND SPECIAL INSPECTION NOTED BELOW. NOTIFY EACH INSPECTION OR TESTING AUTHORITY OR AGENCY 24 HOURS IN ADVANCE OF EACH INSPECTION OR TEST. SUBMIT ONE COPY OF EACH REPORT OR TEST AS IT IS MADE AVAILABLE TO THE ARCHITECT FOR THEIR REVIEW.

3. THE GENERAL CONTRACTOR MUST HIRE AND PAY A THIRD PARTY COMPANY TO COMPLETE ALL REQUIRED INSPECTIONS, TESTING AND SPECIAL INSPECTIONS.

SPECIAL INSPECTION	FREQUENCY	REFERENCED STANDARD
SOIL AND FOUNDATION:		
1. PRIOR TO THE PLACEMENT OF PREPARED FILLS, VERIFY THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.	CONTINUOUS	GEOTECHNICAL ENGINEERING REPORT; IBC 1704.7
2. DURING PLACEMENT AND COMPACTION OF THE FILL MATERIAL, VERIFY THAT THE MATERIAL BEING USED AND MAXIMUM LIFT THICKNESS COMPLY WITH THE GEOTECHNICAL REPORT.	CONTINUOUS	
3. VERIFY, AT THE FREQUENCY SPECIFIED IN THE GEOTECHNICAL REPORT, THAT THE IN-PLACE DRY DENSITY OF COMPACTED FILL COMPLIES WITH THE GEOTECHNICAL REPORT.	CONTINUOUS	
4. VERIFY ALLOWABLE SOIL BEARING CAPACITY.	CONTINUOUS	

SPECIAL INSPECTION	FREQUENCY	REFERENCED STANDARD
CONCRETE:		
1. INSPECTION OF REINFORCING STEEL, SIZE AND PLACEMENT	PERIODIC	ACI 318: 3.5, 7.1-7.7 IBC 1903.5, 1907.1, 1907.7, 1914.4
2. INSPECTION OF REINFORCING STEEL WELDING:		AWS D1.4; ACI 318: 3.5.2 IBC 1903.5.2
a. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A706	PERIODIC	
b. SHEAR REINFORCEMENT	CONTINUOUS	
c. OTHER REINFORCING STEEL	PERIODIC	
3. INSPECT BOLTS TO BE INSTALLED IN CONCRETE, PRIOR TO AND DURING PLACEMENT OF CONCRETE.	CONTINUOUS	IBC 1912.5
4. VERIFYING USE OF REQUIRED DESIGN MIX	PERIODIC	ACI 318: Ch. 4, 5.2-5.4 IBC 1904,1905.2-1905.4,1914.2,1914.3
5. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE TEMPERATURE OF CONCRETE	CONTINUOUS	ASTM C 172; ASTM C 31; ACI 318: 5.6, 5.8 IBC 1905.6,1914.10
6. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	CONTINUOUS	ACI 318: 5.9, 5.10 IBC 1905.9,1905.10, 1914.6-1914.8
7. INSPECTION OF MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	ACI 318: 5.11-5.13 IBC 1905.11,1905.13,1914.9

SPECIAL INSPECTION	FREQUENCY	REFERENCED STANDARD
ADHESIVE ANCHORS/REINF.:		
1. DURING PLACEMENT OF ADHESIVE ANCHORS OR REINFORCEMENT EMBEDDED WITH ADHESIVE (AS SPECIFIED ON THE CONSTRUCTION DOCUMENTS) IN MASONRY AND CONCRETE:		
a. SIZE AND EMBEDMENT OF ANCHORS/REINF.	CONTINUOUS	MANUFACTURERS INSTALLATION INSTRUCTIONS
b. ANCHORS/REINF. INSTALLED PER MANUFACTURERS RECOMMENDATIONS.	CONTINUOUS	

SPECIAL INSPECTION	FREQUENCY	REFERENCED STANDARD
INSPECTION OF FABRICATOR(S):	PERIODIC	IBC 1704.2
1. APPLICABLE ELEMENT (FABRICATOR CERTIFICATION REQUIREMENTS):		
a. STRUCTURAL STEEL (AISC CERTIFIED FOR CONVENTIONAL STEEL BUILDING) b. STEEL JOISTS/JOIST GIRDERS (SJI MEMBER) c. STEEL ROOF DECK (SDI MEMBER) d. PRE-FAB WOOD TRUSSES		
2. WHEN SPECIAL INSPECTIONS ARE REQUIRED BY BUILDING OFFICIAL:		
a. FABRICATION AND IMPLEMENTATION PROCEDURES: THE SPECIAL INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION, CONTROL OF THE WORKMANSHIP, AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK.		
3. WHEN SPECIAL INSPECTIONS ARE NOT REQUIRED BY THE BUILDING OFFICIAL:		
a. UPON COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.		

SPECIAL INSPECTION	FREQUENCY	REFERENCED STANDARD
STEEL CONSTRUCTION:		
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS:		
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	APPLICABLE ASTM MATERIAL SPECIFICATION: AISC 335 Sec. A3.4;AISC LRFD SECTION A3.3
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED	PERIODIC	
2. INSPECTION OF HIGH-STRENGTH BOLTING FOR BEARING-TYPE AND SLIP-CRITICAL CONNECTIONS	CONTINUOUS	AISC LRFD Sec. M2.5 IBC 1704.3.3
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL:		ASTM A-6 OR ASTM A-568 IBC 1708.4
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.		
b. MANUFACTURER'S CERTIFIED MILL TEST REPORTS		
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:		
a. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	AISC ASD, Sec. A3.6 AISC LRFD, Sec.A3.5
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED	PERIODIC	
5. INSPECTION OF WELDING:		
a. STRUCTURAL STEEL:		
1) COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	CONTINUOUS	AWS D1.1 IBC 1704.3.1
2) MULTI-PASS FILLET WELDS	CONTINUOUS	
3) SINGLE-PASS FILLET WELDS > 5/16 "	CONTINUOUS	
4) SINGLE-PASS FILLET WELDS =< 5/16 "	PERIODIC	
5) FLOOR AND ROOF DECK WELDS	PERIODIC	AWS D1.3
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS:	PERIODIC	IBC 1704.3.2
a. DETAILS SUCH AS BRACING AND STIFFENING		
b. MEMBER LOCATIONS		
c. APPLICATION OF JOINT DETAILS AT EACH CONNECTION		

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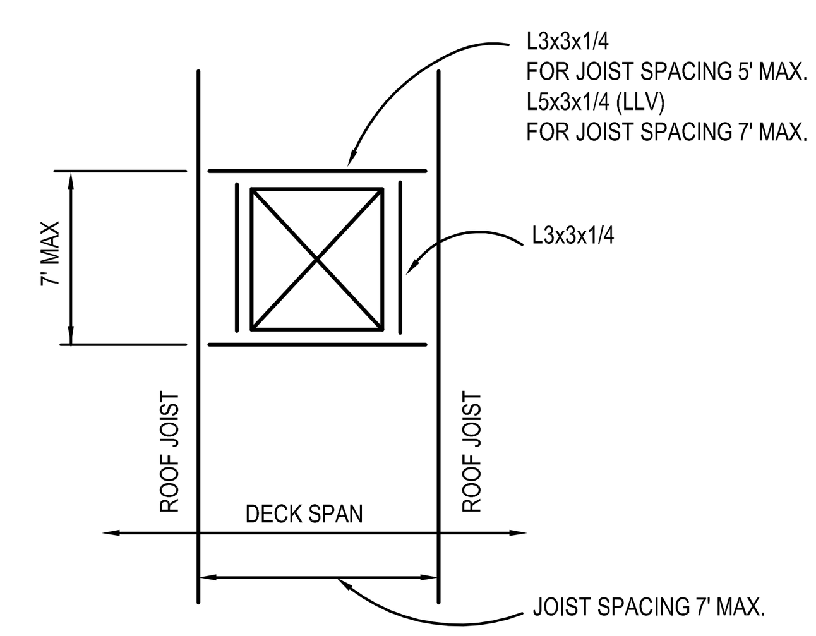
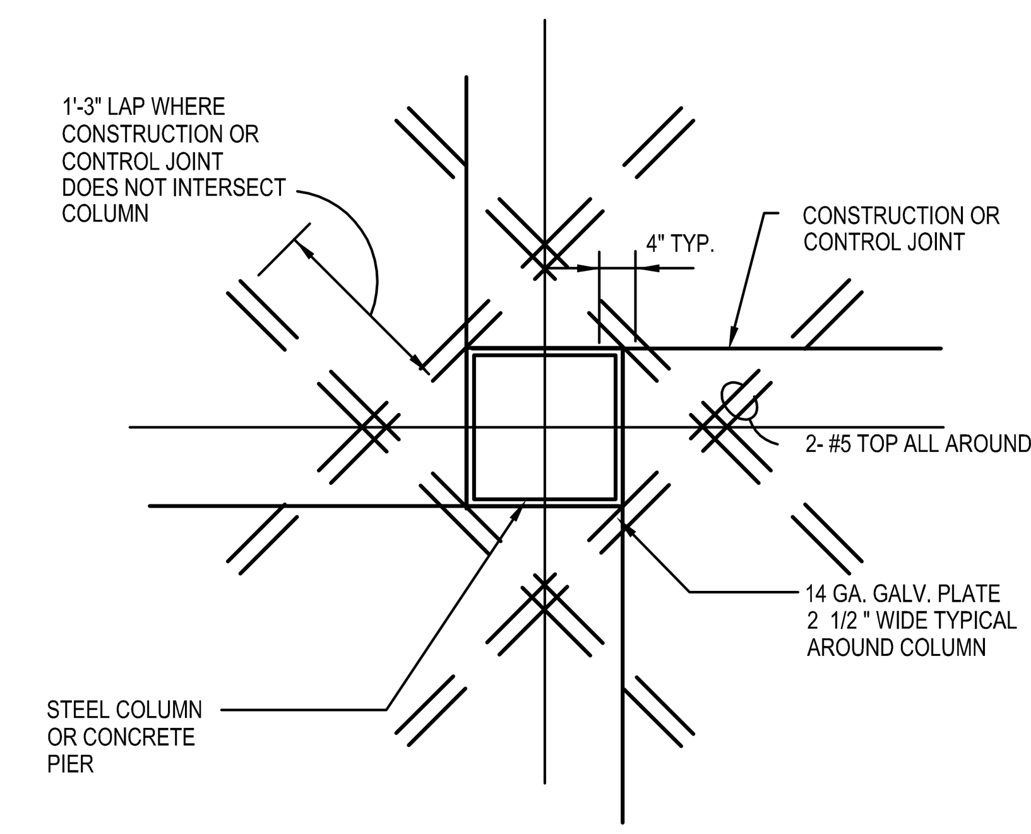
FOOTING SCHEDULE (ASSUMED NET ALLOWABLE SOIL PRESSURE = 2000 psf)			
FOOTING MARK	FOOTING SIZE	FOOTING REINFORCEMENT	REMARKS
F4.0	4'-0"x4'-0"x1'-4"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	
F5.0	5'-0"x5'-0"x1'-4"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	
F5.5	5'-6"x5'-6"x1'-4"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	
F6.0	6'-0"x6'-0"x1'-4"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	
F7.0	7'-0"x7'-0"x1'-6"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	
F8.0	8'-0"x8'-0"x1'-6"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	
F4.0x13.0	4'-0"x13'-0"x1'-4"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	
F4.0x18.0	4'-0"x18'-0"x1'-4"	#6 @ 12" O.C. EACH WAY - TOP & BOTTOM	

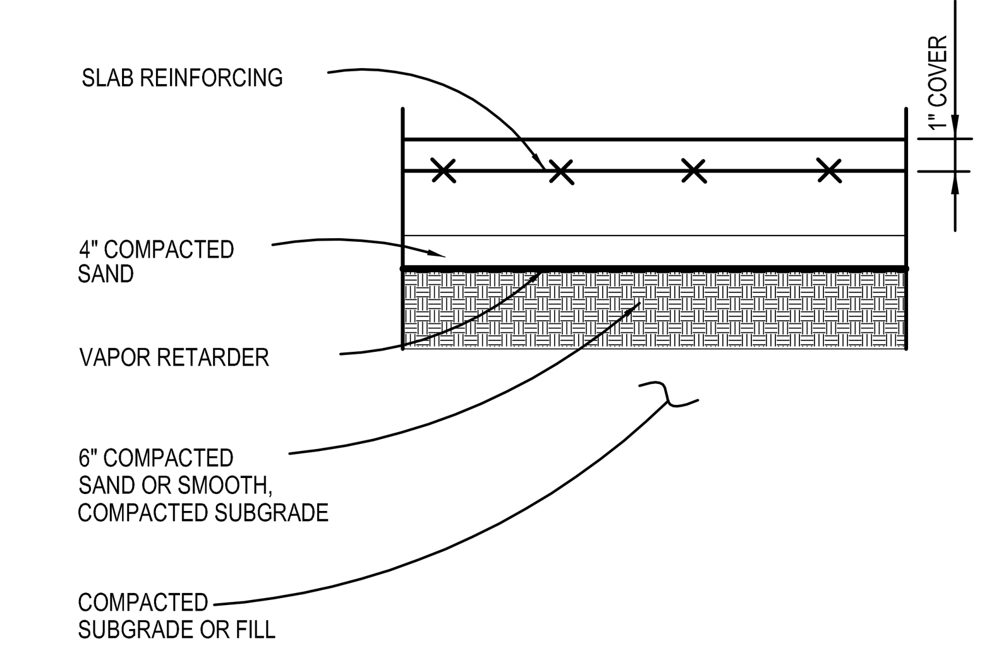
STEEL COLUMN SCHEDULE				
COLUMN MARK	COLUMN SIZE	BASE PLATE SIZE BxNxt	ANCHOR BOLTS SIZE	EMBEDMENT LENGTH
C-1	HSS8x8x0.5	15"x15"x1 1/2"	4-1 1/4" DIA.	1'-3"
C-2	HSS8x8x0.1875	15"x15"x3/4"	4-3/4" DIA.	1'-0"
C-3	HSS8x8x0.25	15"x15"x1 1/4"	4-1 1/4" DIA.	1'-3"
C-4	HSS8x8x0.25	15"x15"x1 1/4"	4-1 1/4" DIA.	1'-3"
C-5	HSS8x8x0.25	15"x15"x1 1/4"	4-1 1/4" DIA.	1'-3"
C-6	HSS8x8x0.5	15"x15"x1 1/2"	4-1 1/4" DIA.	1'-3"
C-7	HSS8x8x0.5	15"x15"x1 1/2"	4-1 1/4" DIA.	1'-3"
C-8	HSS8x8x0.5	15"x15"x1 1/2"	4-1 1/4" DIA.	1'-3"
C-9	HSS8x8x0.1875	15"x15"x3/4"	4-3/4" DIA.	1'-0"
C-10	HSS8x8x0.25	15"x15"x1 1/2"	4-1 1/4" DIA.	1'-3"
C-11	HSS8x8x0.5	15"x15"x1 3/4"	4-1 1/2" DIA.	1'-3"
C-12	HSS8x8x0.5	15"x15"x1 3/4"	4-1 1/2" DIA.	1'-3"
C-13	HSS8x8x0.5	15"x15"x1 3/4"	4-1 1/2" DIA.	1'-3"
C-14	HSS8x8x0.5	15"x15"x1 3/4"	4-1 1/2" DIA.	1'-3"
C-15	HSS8x8x0.5	15"x15"x1 3/4"	4-1 1/2" DIA.	1'-3"
C-16	HSS8x8x0.25	15"x15"x1 3/4"	4-1 1/2" DIA.	1'-3"
C-17	HSS8x8x0.1875	15"x15"x3/4"	4-3/4" DIA.	1'-0"
C-18	HSS8x8x0.625	15"x15"x1 3/4"	4-1 1/2" DIA.	1'-3"
C-19	HSS8x8x0.625	15"x15"x1 3/4"	2-1 1/2" DIA. ANCH. BOLTS + 2-1 1/2" EXP. ANCH.	1'-3"
C-20	HSS6x6x0.625			



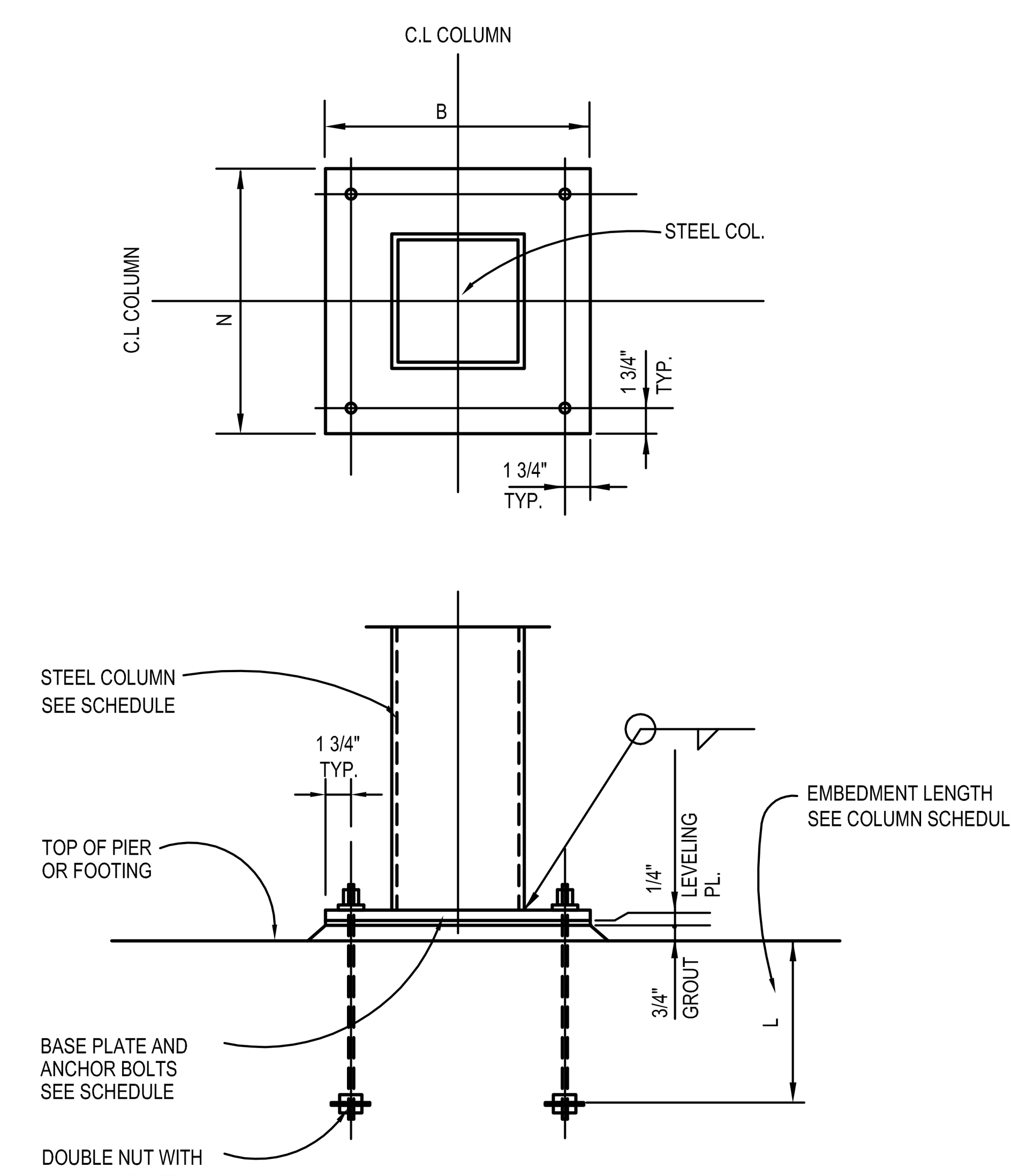
7 S2.3 TYPICAL ROOF DECK SUPPORT AT OPENING



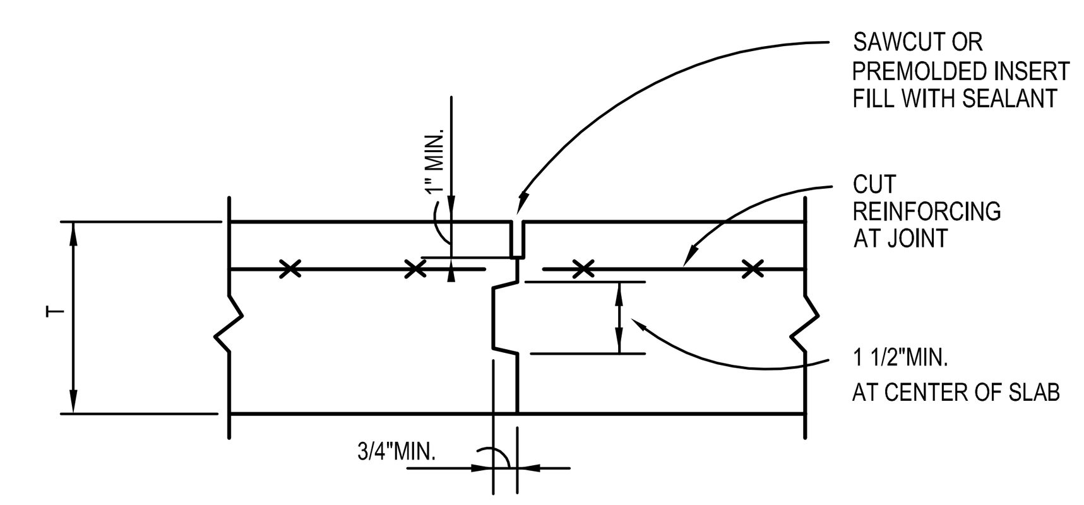
5 S2.3 TYPICAL ISOLATED TUBE STEEL COLUMN - SLAB ON GRADE



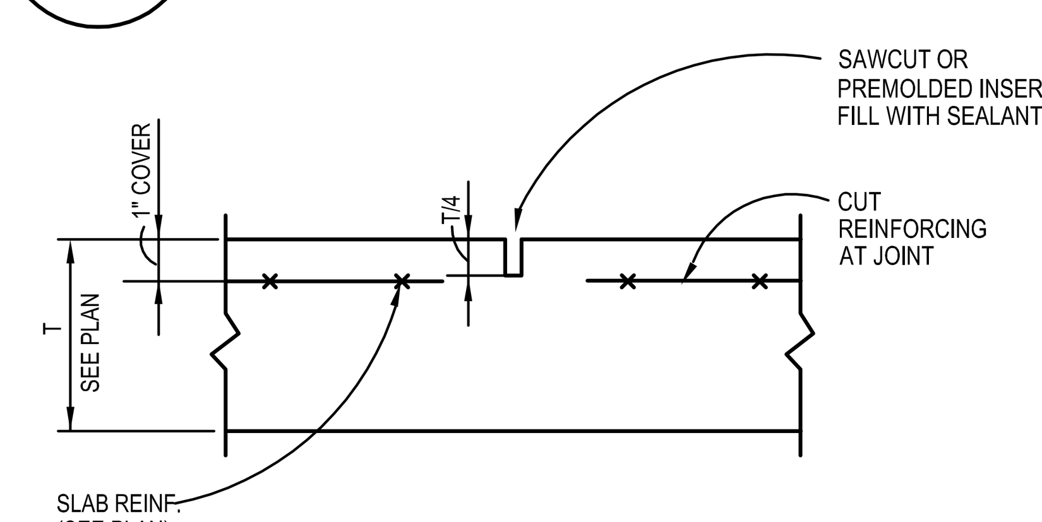
3 S2.3 TYPICAL SLAB ON GRADE



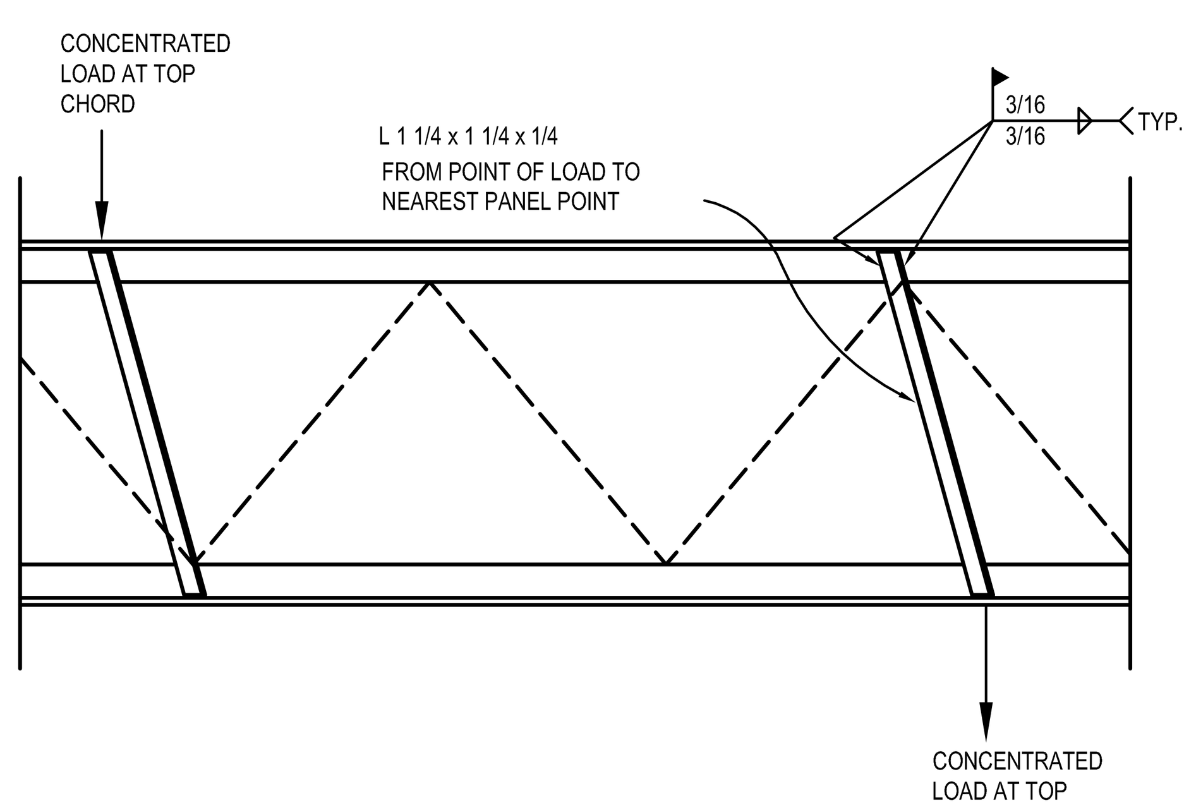
4 S2.3 TYPICAL COLUMN BASE PLATE DETAIL



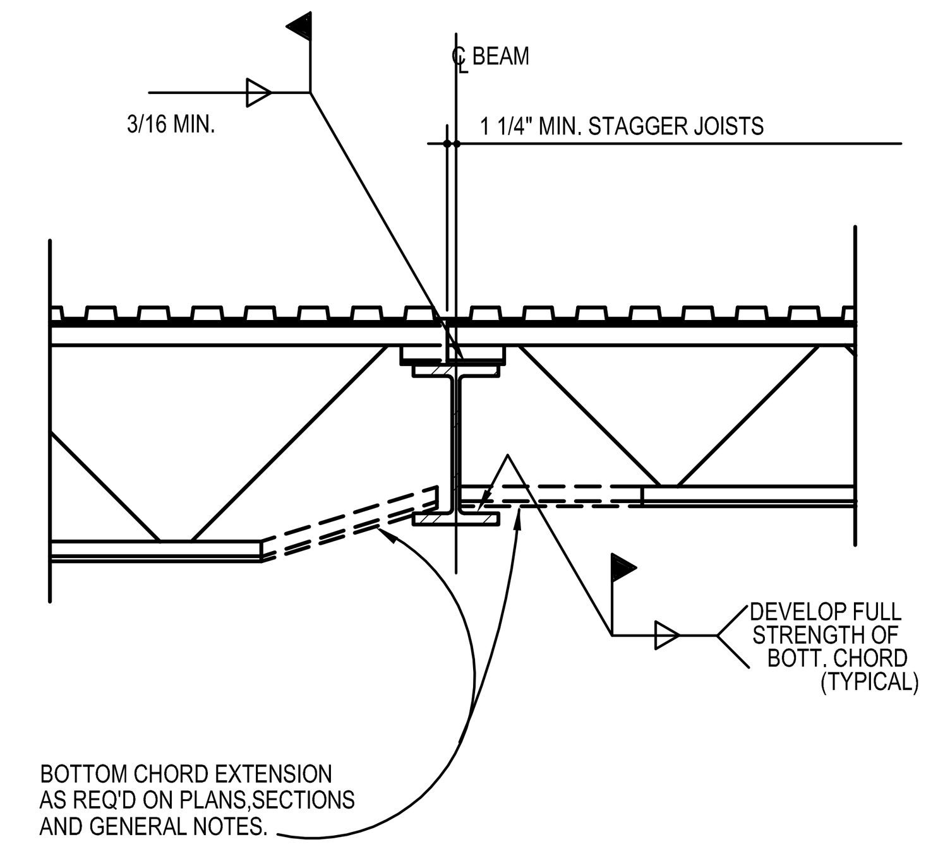
2 S2.3 TYPICAL CONSTRUCTION JOINT



1 S2.3 TYPICAL CONTROL JOINT



8 S2.3 TYPICAL JOIST REINFORCEMENT @ CONCENTRATED LOADS



6 S2.3 TYPICAL JOIST BEARING ON STEEL BEAMS

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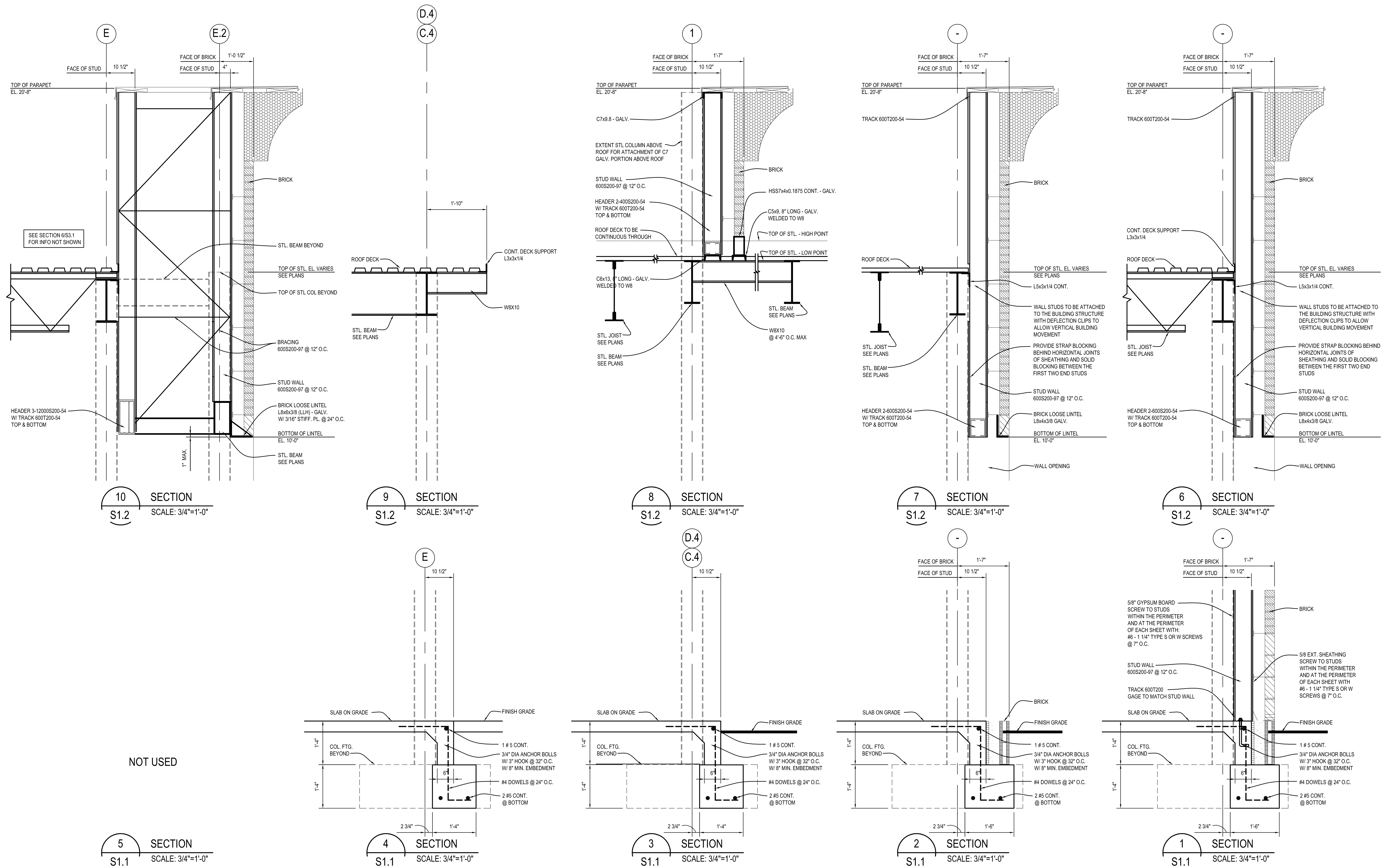

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SHEET NUMBER
S3.1